TOWN OF PENDLETON
6570 Campbell Boulevard
Lockport, NY 14094

Deborah K. Maurer, Town Clerk
Phone: (716) 625-8833
Fax: (716) 625-6295
dmaurer@pendletonny.us

TOWN OF PENDLETON
NOTICE OF PUBLIC HEARING

NOTICE IS HEREBY GIVEN that a Public Hearing will be held by the ZONING BOARD OF APPEALS, Town of Pendleton, at the Town Hall, 6570 Campbell Boulevard, Lockport, NY 14094, at 7:00 p.m. on Tuesday, the 26th day of August 2025 for:

Dale and Wendy Harman
5675 Dunnigan Road
Lockport, NY 14094

Owner wishes to construct an accessory structure on the above referenced property with a 10 ft. side yard setback. Town Code requires a minimum 15 ft. side yard setback. An area variance of 5 ft. is being requested.

Town Ordinance Affected: § 247-34F(3)(b)
Variance Sought: 5 Ft. Distance Variance
Size of Parcel: 150' x 755'
Current Zoning: R-2 Residential

Additional information pursuant to this public hearing may be available at https://pendletonny.us/calendar-events/.

Deborah K. Maurer
Deborah K. Maurer, Town Clerk

Dated: August 14, 2025

Please Publish: August 19, 2025

Town of Pendleton
Building Department

6570 Campbell Blvd.
Lockport NY 14094
716-625-8833 Ext. 115 & 114
www.pendletonny.us
rdiedrich@pendletonny.us
tbarnes@pendletonny.us

PUBLIC HEARING REQUEST TO PETITION FOR A VARIANCE

Fee: $125.00

TO BE HELD BY: Zoning Board of Appeals

DATE OF HEARING: Aug 26 2025 TIME: 7:00 P.M.

REQUESTED BY: RJKing builders PHONE: [redacted]

ADDRESS OF PROPERTY: 5675 Dunnigan Rd.

ADDRESS OF OWNER: 5675 Dunnigan Rd.

E-MAIL ADDRESS: [redacted]

To Consider the Following Request: TO CONSTRUCT AN ACCESSORY STRUCTURE WITH A 10 FT SIDE YARD SETBACK. REQUIRED PER CODE IS 15FT. SEEKING 5FT VARIANCE.

Town Ordinances Affected: 247-34 F(3)(b) SIDE YARD SETBACK

Variance Sought: 5FT.

Size of Parcel: 150' x 755'

Current Zoning: R-2 RESIDENTIAL

(RJKing LLC) July 25th 2025

Applicant Signature Date

Notes: This form must be filed with the Town Clerk, along with the filing fee, 10 days before the date of the Public Hearing. Please bring all documents to the Public Hearing including, Denial for Building Application, Site Survey showing proposed variance, documented reasons for the benefit of relief and any other documents that will support your need for relief. Structures over 1800 SF require a full site plan review by the Town Planning Board, per Town Code §247.34.F (4).

Town of Pendleton
Building Department

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716-625-8833 Ext. 115 & 114
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DENIAL OF BUILDING APPLICATION

PROPERTY LOCATION: 5675 Dunnigan Rd.

SBL NUMBER: 151.00-1-42.12

OWNER: Dale + Wendy Herman

OWNER ADDRESS: 5675 Dunnigan Rd.

E-MAIL ADDRESS: [redacted]

REASON FOR DENIAL

PROPOSED ACCESSORY STRUCTURE DOES NOT MEET REQUIRED SIDE YARD SETBACK PER TOWN CODE 247-34 F(3)(b) 15 FT. SIDE YARD SETBACK

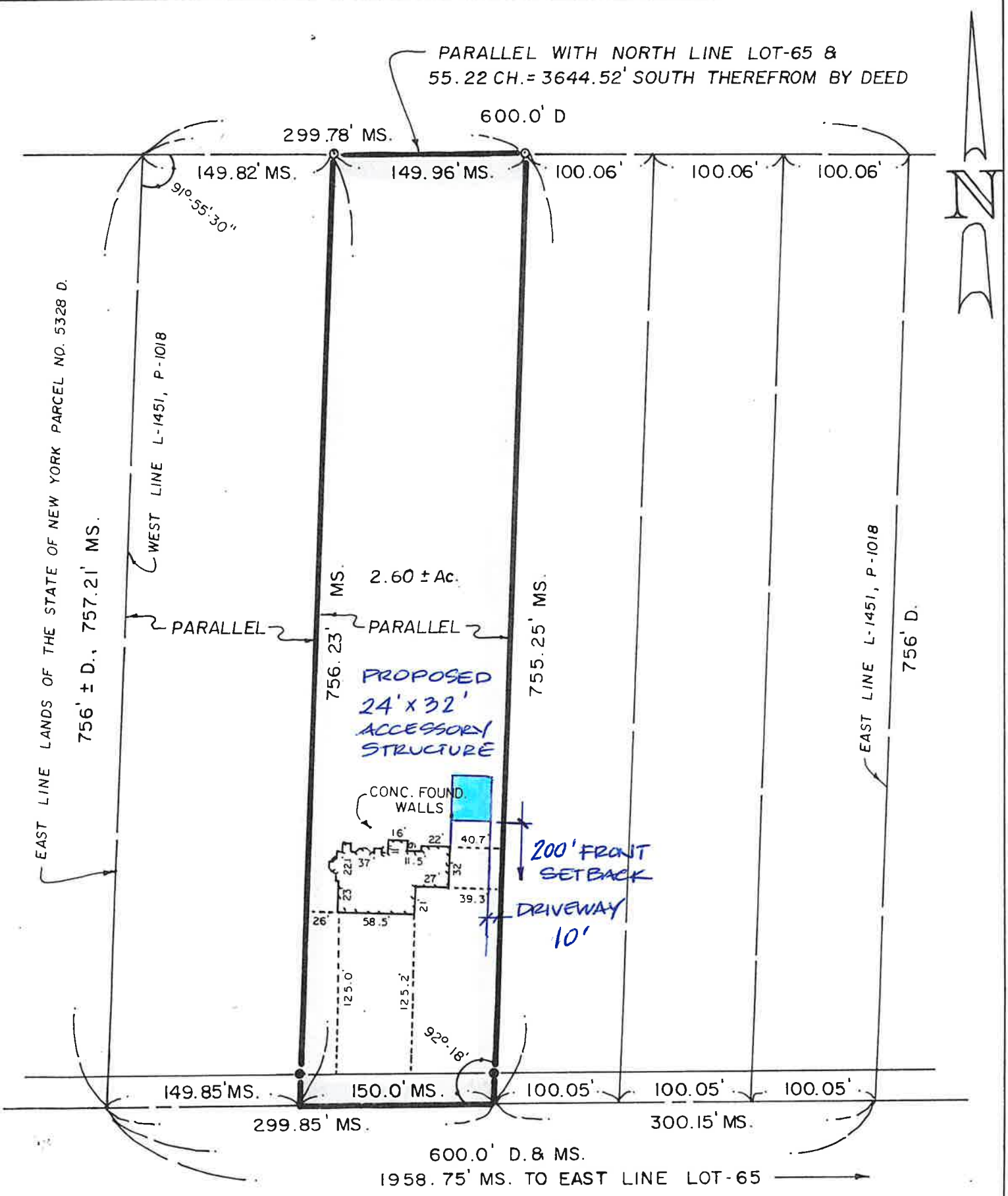
PROPOSED SIDE YARD SETBACK IS 10'

NOTE: This form and supporting documentation must be filed with the Board of Appeals

Applicant — Date: July 25 2025

Code Enforcement Officer — Date: 8/12/2025

Revised 1/14/2025



tbarnes@pendletonny.us

From: rboking@aol.com
Sent: Friday, July 25, 2025 11:48 AM
To: TBarnes@Pendletonny.us
Subject: Fw: 5675 Dunnigan Pole Barn

----- Forwarded Message -----
From: [redacted]>
To: "rboking@aol.com" <rboking@aol.com>
Cc: Dale Harman [redacted]
Sent: Friday, July 25, 2025 at 11:36:25 AM EDT
Subject: 5675 Dunnigan Pole Barn

From: Wendy and Dale Harman
5675 Dunnigan Rd.
Pendleton, NY 14094

To whom it may concern:

We are planning on having a pole barn built on our property in September, 2025.

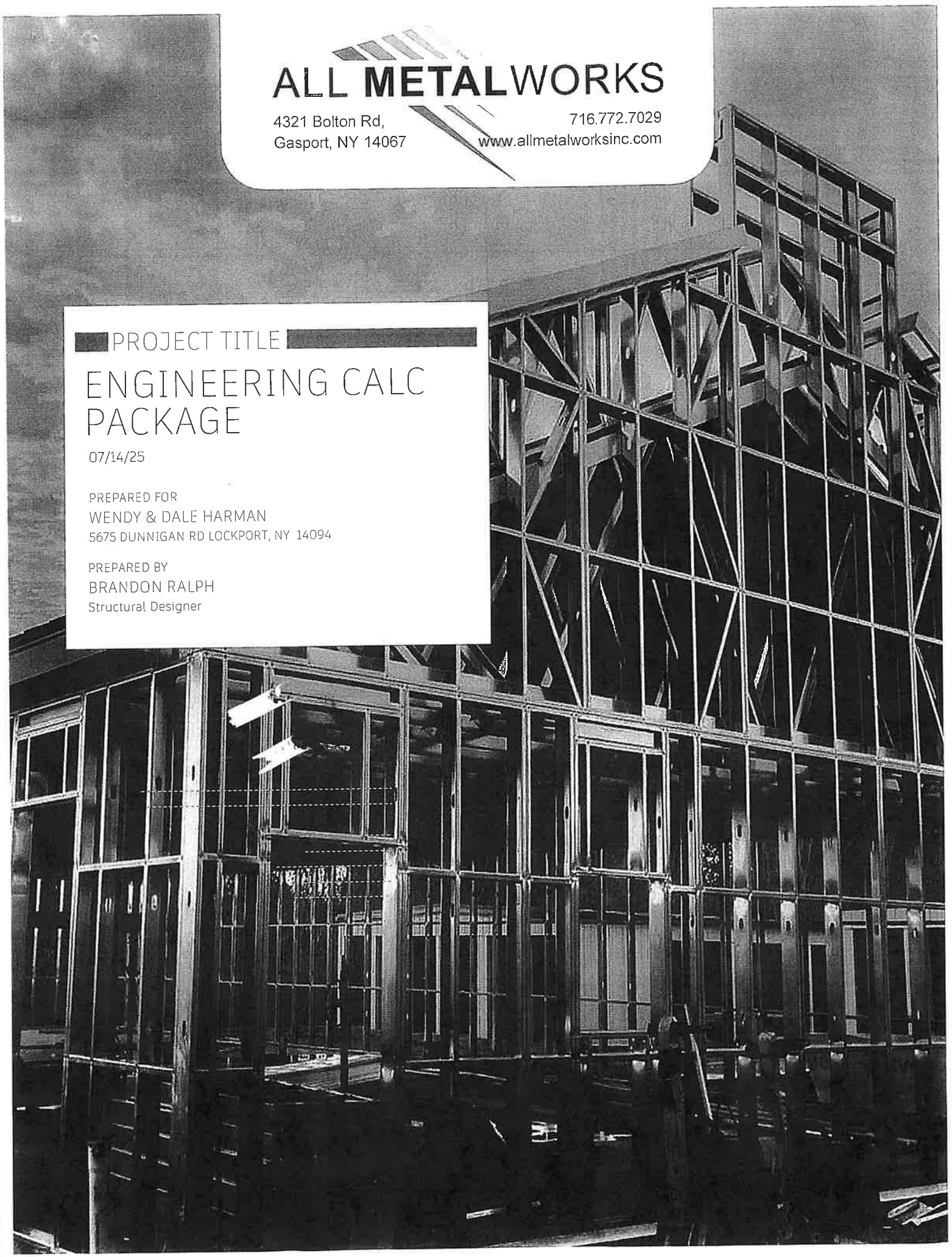
RJBuilders is our contractor.

RJKing Builders has permission to proceed with the process of working on our behalf to pull permits, obtain variances, etc.

Thank you,

Wendy and Dale Harman

ALL METALWORKS
4321 Bolton Rd,
Gasport, NY 14067
716.772.7029
www.allmetalworksinc.com



PROJECT TITLE

ENGINEERING CALC PACKAGE

07/14/25

PREPARED FOR
WENDY & DALE HARMAN
5675 DUNNIGAN RD LOCKPORT, NY 14094

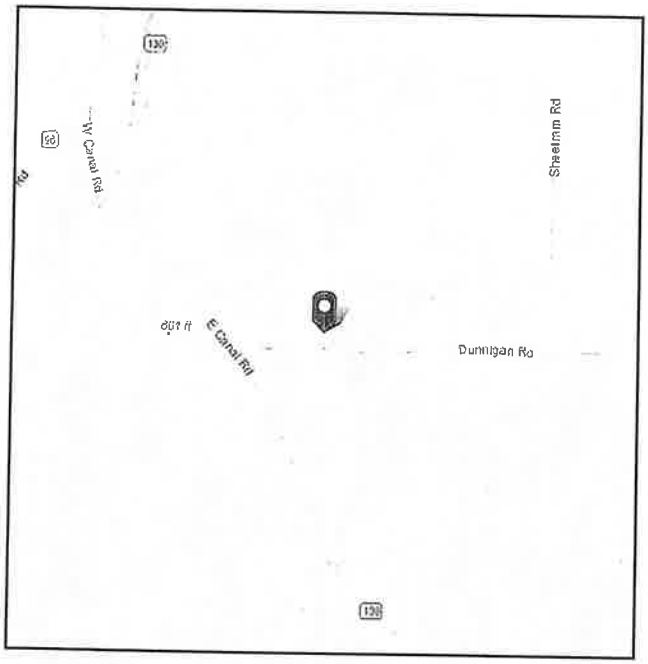
PREPARED BY
BRANDON RALPH
Structural Designer

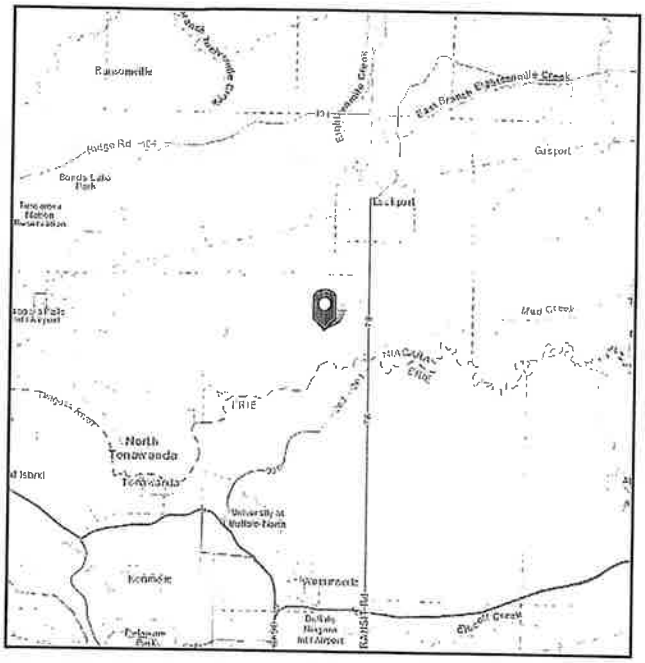
ASCE Hazards Report

Address:
5675 Dunnigan Rd
Lockport, New York
14094

Standard: ASCE/SEI 7-22
Risk Category: II
Soil Class: D - Stiff Soil

Latitude: 43.102104
Longitude: -78.731555
Elevation: 585.8260314662487 ft (NAVD 88)





Wind

Results:

Wind Speed	109 Vmph
10-year MRI	75 Vmph
25-year MRI	81 Vmph
50-year MRI	86 Vmph
100-year MRI	92 Vmph
300-year MRI	102 Vmph
700-year MRI	109 Vmph
1,700-year MRI	116 Vmph
3,000-year MRI	121 Vmph
10,000-year MRI	132 Vmph
100,000-year MRI	152 Vmph
1,000,000-year MRI	173 Vmph

Data Source: ASCE/SEI 7-22, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2
Date Accessed: Fri Jul 18 2025

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Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-22 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years). Values for 10-year MRI, 25-year MRI, 50-year MRI and 100-year MRI are Service Level wind speeds, all other wind speeds are Ultimate wind speeds.

Site is not in a hurricane-prone region as defined in ASCE/SEI 7-22 Section 26.2.

APEX CONSULTING
Survey & Engineering Services, P.C.
102 East Avenue
Lockport, NY 14094
Office:(716)439-0188 • Fax:(716)439-0189

JOB NO. 25-046
7/23/2025

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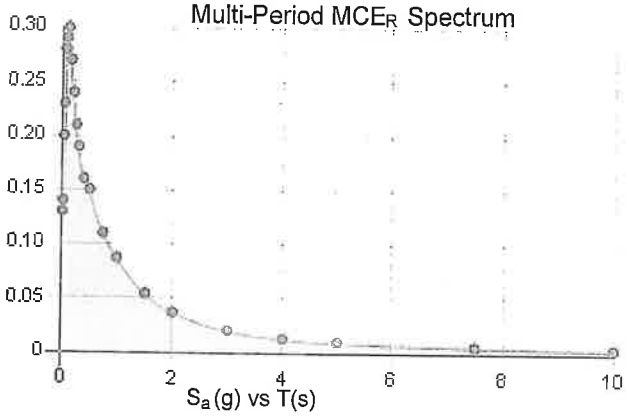
Seismic

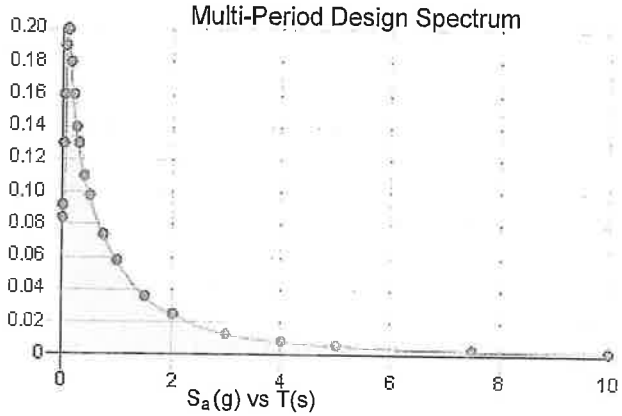
Site Soil Class: D - Stiff Soil

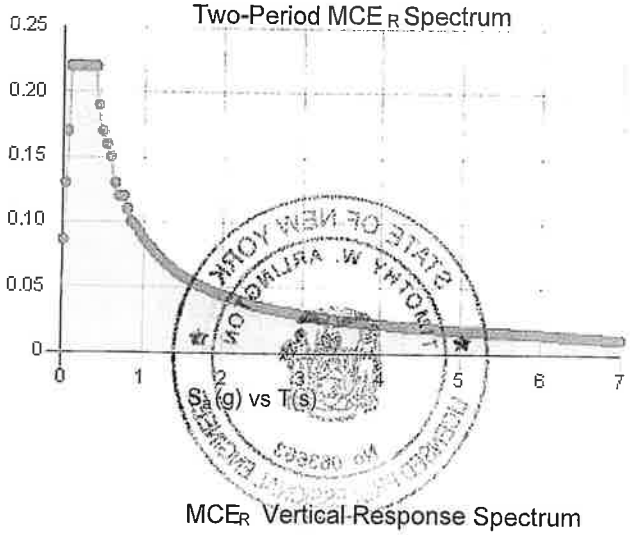
Results:

PGA_M :	0.11	T_L :	6
S_{MS} :	0.22	S_S :	0.18
S_{M1} :	0.087	S_1 :	0.041
S_{DS} :	0.14	V_{S30} :	260
S_{D1} :	0.058		

Seismic Design Category: A

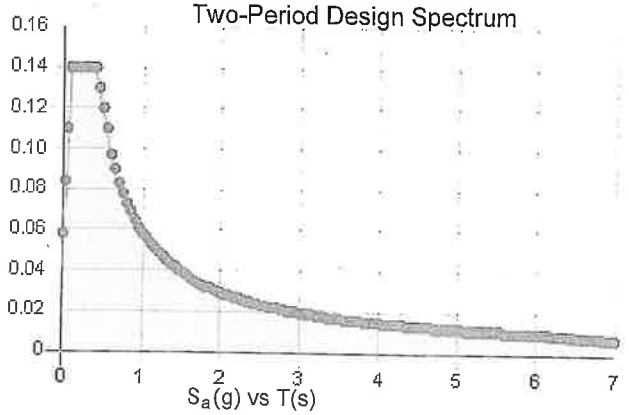






MCER Vertical Response Spectrum

Vertical ground motion data has not yet been made available by USGS.



Design Vertical Response Spectrum

Vertical ground motion data has not yet been made available by USGS.

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Data Accessed: Fri Jul 18 2025

Date Source:
USGS Seismic Design Maps based on ASCE/SEI 7-22 and ASCE/SEI 7-22 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-22 Ch. 21 are available from USGS.

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AMERICAN SOCIETY OF CIVIL ENGINEERS

The ASCE Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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Connection	Section	Length	Axial	Int.	Fastener	Pa	Req.
T1-T2	Plate - 0.0451"	0.00	2.01C	0.00	#10-16 FPHSD	0.287	8
TC/BC(Knee L)	Plate - 0.0451"	0.00	1.18T	0.00	#10-16 FPHSD	0.287	8
TC/BC(Knee R)	Plate - 0.0451"	0.00	1.18T	0.00	#10-16 FPHSD	0.287	8
BC #1	600S162-43(50)	25.82	1.03C	0.78	#10-16 FPHSD	0.000	0
TC #1	600S162-43(50)	16.80	2.01C	0.50	#10-16 FPHSD	0.000	0
TC #2	600S162-43(50)	16.80	2.01C	0.50	#10-16 FPHSD	0.000	0
Web # 1 7	600S162-43(50)	0.97	0.87C	0.14	#10-16 FPHSD	0.287	4
Web # 2 6	600S162-43(50)	7.69	1.00C	0.50	#10-16 FPHSD	0.287	4
Web # 3 5	600S162-43(50)	7.93	0.72C	0.37	#10-16 FPHSD	0.287	4
Web # 4	600S162-43(50)	10.76	1.50T	0.29	#10-16 FPHSD	0.287	6
BC Lateral Brace	150F125-30(33)	4.00	0.22C	0.17	#10-16 FPHSD	0.160	2
BC Diagonal Brace	150F125-30(33)	8.94	0.49C	0.74	#10-16 FPHSD	0.160	4

Connection	Simpson	each	Load	Uplift/Shr	Fastener	Pa	Req.
Chord-Wall				0.72	#10-16 FPHSD	0.287	3
Truss Chord	S/H1	1	0.01		#10-16 FPHSD	0.287	3
Steel Stud				0.01	#10-16 FPHSD	0.287	3
Truss Chord	S/H1	1	0.01		#10-16 FPHSD	0.287	3
Steel Stud				0.01	#10-16 FPHSD	0.287	3

GENERAL NOTES

1. Trusses require lateral bracing. See Truss Layout and Detail sheets.
2. Top Chord continuously sheathed.
3. Number of fasteners noted in chart installed on each end of Web
4. Allowable fastener values based on Simpson Report ESR-3558.
5. (ws) denotes web stiffener required at support.
6. Member design based on sections in Howick560FS Library.

Maximum Deflections

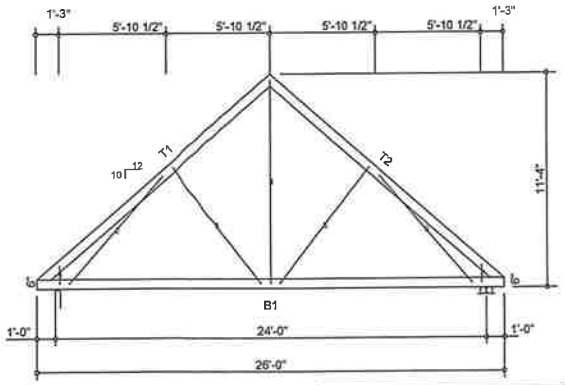
Vertical	0.123 in	(L / 2310)
Horizontal	0.037 in	
Bottom Overhang	0.014 in	(L / 993)
Vertical	0.088 in	(L / 3229) [Dead Load Only]
Vertical	0.031 in	(L / 9165) [Live Load Only]

Support Reactions	Down	Uplift*	Horizontal	Bearing
Left	2.50 (2.50)	-0.01 [-1.35]	0.72	3.50
Right	2.50 (2.50)	-0.01 [-1.35]	0.00	3.50

* Uplift Load Combination (Truss to Support Connection Only): 0.6Dead + 0.6Wind
( ) Denotes 'Dead+Live Only/Snow Only'
[ ] Denotes 'Wind Only' Uplift Reaction

DESIGN DATA

Number of Trusses = 7 each
Plate Style : Out-Of-Plane
Eave Height : 14.00 ft (top of wall)
Bearing : 6 in
Spacing : 4.00 ft
Dead Load : 10.00 psf (top chord)
Dead Load : 10.00 psf (bottom chord)
Live Load : 20.00 psf (top chord)
Live Load : 0.00 psf (bottom chord)
Snow Load : 50.00 psf (ground) [Ct =1.00]
Snow Load : 23.78 psf (design) [Is =1.00, Ce = 0.90]
Wind Load : 25.79 psf (design)
Wind Speed : 115 mph (Exposure C)
Open Category: E
Topography (Kz):1
Building Category: (2) General
Seismic Coefficient: 0.0



Per AISI S100-16		Actual			Allowable			
Member	Section	Po	Vo	Mo	Pa	Va	Ma	Ratio
Bottom Chord	1-600S162-43(50)	1.03T	0.98	8.80	13.38	1.43	22.96	0.78
Top Chord	1-600S162-43(50)	2.01C	0.38	5.13	7.19	1.43	22.96	0.50
Web	1-600S162-43(50)	1.00C	0.00	0.38	2.02	1.43	3.12	0.50

Truss Design Code: AISI S240-15
International Building Code 2018: PASSED
Design Method - (ASD)
Main Wind Pressure Design

All Metal Structures

4321 Bolton Rd
Gasport, NY 14094
(716) 772-7029

5675 Dunnigan Rd

Lockport, NY 14094

Truss D&E, V30.00

Designer: BR
File: T1
Job Number: —

T1

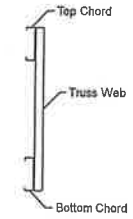
Design Dwg 1/1

MATERIAL CUT LIST	Material	Unit/Length	Quantity
Metal Screw	#10-16 FPHSD	0	1119
Connector (P3)	PL18x16x0.045	0	7
Connector (P5)	PL18x21x0.045	0	14
B1	600S162-43(50)	26'-0"	7
T1	600S162-43(50)	16'-11 1/16"	7
T2	600S162-43(50)	16'-11 1/16"	7
Web #1 7	600S162-43(50)	1'-3 3/4"	14
Web #2 6	600S162-43(50)	7'-9 9/16"	14
Web #3 5	600S162-43(50)	8'-3 5/16"	14
Web #4	600S162-43(50)	11'-1 1/4"	7

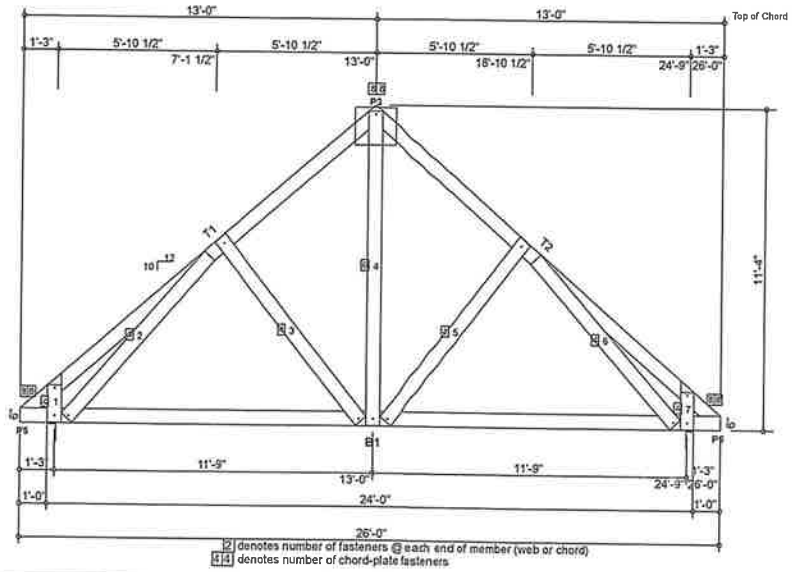
GENERAL NOTES

1. Number of fasteners noted in chart installed on each end of Web
2. When connecting plates to chord or web members, fasteners must pass through the plate and member. If a joint connection has fasteners passing through the web, connector plate and chord, the fastener may be counted as part of the required number of fasteners for that joint.
3. Minimum connector spacing (center to center or free edge) = 5/8"
4. Truss connector type - #10-16 FPHSD (Simpson)

Number of Trusses	= 7 each
Chord Cut Type	= Sq-Mitre-Sq Cut
Truss Spacing	= 4'-0"
Number of Plys	= 1
Weight per Truss	= 174.1 lbs (per ply)
Total Steel	= 105.729 ft (per ply)



Truss Cross-section



2 denotes number of fasteners @ each end of member (web or chord)
4|4 denotes number of chord-plate fasteners

Scale: 3/16" = 1'-0"

All Metal Structures
4321 Bolton Rd
Gasport, NY 14094
(716) 772-7029

5675 Dunnigan Rd

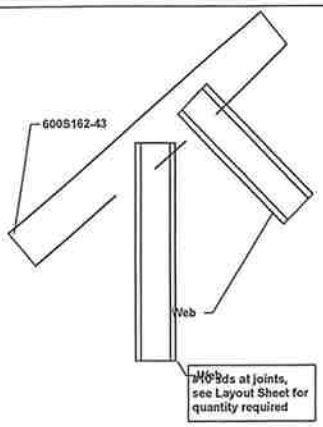
Lockport, NY 14094

Truss D&E, V30.00

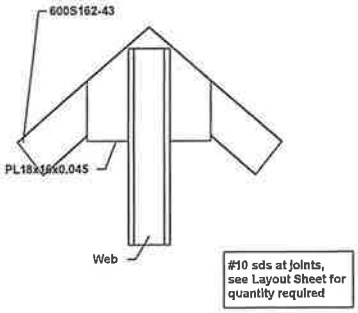
Designer: BR
File: T1
Job Number: ---

T1

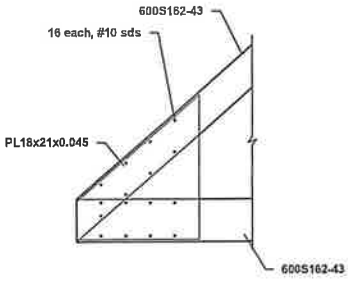
Fabrication Drawing



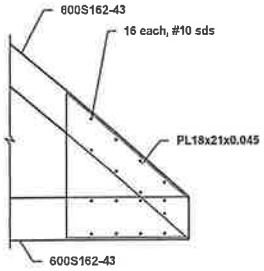
1 T1_1 TRUSS DETAIL



2 T1_1 TRUSS DETAIL

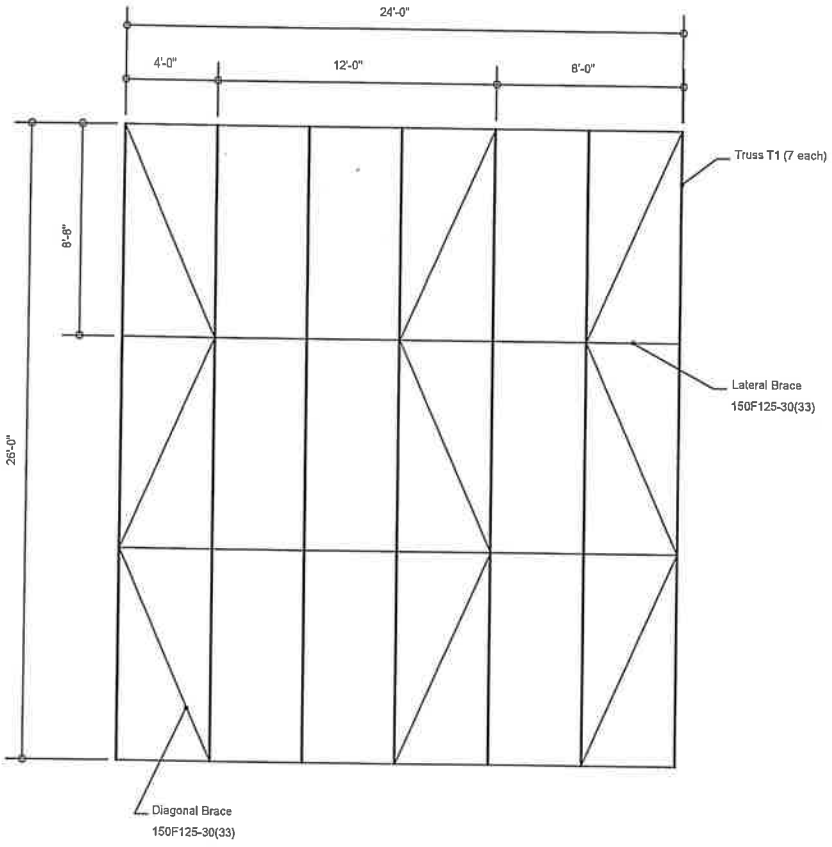


3 T1_1 TRUSS DETAIL

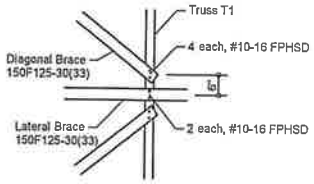


4 T1_1 TRUSS DETAIL

All Metal Structures 4321 Bolton Rd Gasport, NY 14094 (716) 772-7029	**5675 Dunnigan Rd** Lockport, NY 14094	Truss D&E, V30.00 Designer: BR File: T1 Job Number: —	This document contains information proprietary to All Metal Works and may not be used or reproduced without prior written consent. Contents subject to change without notice.	T1_1 Detail Page 1/2



1 T1_2 **BOTTOM CHORD BRACING PLAN**



2 T1_2 **BOTTOM CHORD BRACING PLAN**

All Metal Structures
4321 Bolton Rd
Gasport, NY 14094
(716) 772-7029

5675 Dunnigan Rd

Lockport, NY 14094

Truss D&E, V30.00

Designer: BR
File: T1
Job Number: —

T1_2

All Metal Structures

4321 Bolton Rd
Gasport, NY 14094
(716) 772-7029

Truss D&E, V30.00

D1
File: T1

Project	: 5675 Dunnigan Rd
Location	: Lockport, NY 14094
Job Number	: ---
Designer	: BR

STEEL DESIGN CODE	| AISI S100-16	
Truss Design Code	| AISI S240-15	
Number of Trusses	| 7 each	
Truss Type	| Belgian Truss	
Truss Style	| Out-Of-Plane	
Support Width	| 6.00 in	
Offset Webs	| Yes	
Eave Height	| 14.00 ft	
Truss Length	| 26.00 ft	
Truss Spacing	| 4.00 ft	
Truss Overhang	| 1.00 ft	(Bottom Chord Extension - Left)
Truss Overhang	| 1.00 ft	(Bottom Chord Extension - Right)
Truss Support Type	| Sliding	
Truss Slope (T)	| 10.00:12	
Truss Slope (B)	| 0.00:12	

Dead Load	= 10.0 psf	(Top Chord)		
Dead Load	= 10.0 psf	(Bottom Chord)		
Live Load	= 20.0 psf	(Top Chord)		
Live Load	= 0.0 psf	(Bottom Chord)		
Snow Load	= 50.0 psf	(Ground)	I (snow) = 1.00	
Snow Load	= 23.8 psf	(Design)	Ce(snow) = 0.90	Ct(snow) = 1.00
Wind Load	= 25.8 psf			
Wind Speed	= 115 mph	(Exposure C)	Open Category = E	
Building Category	= (2) General			
Seismic Coefficient	= 0.000			

T1

Member	Section Type	Actual Po kip	Actual Vo kip	Actual Mo kp-in	Allowable Pa kip	Allowable Va kip	Allowable Ma kp-in	Int. Ratio
Bottom Chord	1-600S162-43(50)	1.03T	0.98	8.80	13.38	1.43	22.96	0.78
Top Chord	1-600S162-43(50)	2.01C	0.38	5.13	7.19	1.43	22.96	0.50
Webs	1-600S162-43(50)	0.23T	0.00	0.38	13.38	1.43	3.12	0.50

International Building Code 2018: PASSED

SECTION NOMENCLATURE EXAMPLE - 600S162-43(50) [Howick560FS Library]

- 600 = Section Depth (inches x 100)
- S = Material Type Symbol (See Fy)
- 162 = Flange Width (inches x 100)
- 43 = Thickness (inches x 1000)
- (50) = Fy (yield stress, ksi)

Project : 5675 Dunnigan Rd
Location : Lockport, NY 14094
Truss Mark : T1

D2
File: T1

MAXIMUM SUPPORT REACTIONS

SUPPORT	P(up)	P(dn)	V(<->)
Left	2.50 kip	0.00 kip	0.72 kip
Right	2.50 kip	0.00 kip	0.00 kip

Maximum Vertical Deflection = 0.088 in (L / 3229, between supports [Dead Load Only]
Maximum Vertical Deflection = 0.031 in (L / 9167, between supports [Live Load Only]
Maximum Vertical Deflection = 0.123 in (L / 2310, between supports [Total Load]
Maximum Vertical Deflection = 0.014 in (L / 993, bottom chord overhang [Total Load]
Maximum Horizontal Deflection = 0.037 in (L / 4541, top of wall [Total Load])

WIND LOAD DATA (per ASCE 7-16)

Basic Wind Speed(V)	=115 mph	
Air Density(Vc)	= 0.00256	
Direction Factor(Kd)	=0.85	'ASCE 7-16 [Table 26.6-1]
Exposure Factor(Kz)	=0.90	'ASCE 7-16 [Table 26.10-1, note 1]
Topography Factor(Kzt)	=1.00	
Gust Factor(Gz)	=0.85	'ASCE 7-16, Eq. C5
Basic Wind (Qz)	= 25.79 psf	'Qz = Vc*Kz*Kzt*Kd*Ke*V^2, ASCE 7-16, Eq. 26.10-1

MAIN FRAME EXTERNAL PRESSURE COEFFICIENTS

Windward Wall Pressure	= 22.00 psf	Cq = 0.86	Cq = [GzCp + GCpi] GCpi = +-0.18
Leeward Wall Pressure	= -16.00 psf	Cq =-0.61	h = 25.27 ft
Roof LWN Pressure	= -17.75 psf	Cq =-0.69	h/L = 0.97
Roof WWN Pressure(o)	= -8.00 psf	Cq =-0.27	Ar = 104 ft^2
Roof WWN Pressure(i)	= 10.00 psf	Cq = 0.39	
Roof LWP Pressure	= -24.25 psf	Cq =-0.95	
Roof WWP Pressure	= -24.25 psf	Cq =-0.95	
Overhang Pressure(BC)	= 22.00 psf	Cq = 0.86	

TRUSS DESIGN CHART (T1)

Member	Component Type	Length ft	P kip	Mec kp-in	Rm	Int. Ratio	Fastener Type	Va kip	Vmax kip	Qty
T1-T2	Plate - 0.0451"	--	2.01C	---	---	---	#10-16 FPHSD	0.287	2.01C	8
TC/BC(Knee L)	Plate - 0.0451"	--	1.18T	---	---	---	#10-16 FPHSD	0.287	1.18T	8
TC/BC(Knee R)	Plate - 0.0451"	--	1.18T	---	---	---	#10-16 FPHSD	0.287	1.18T	8
BC #1	600S162-43(50)	25.82	1.03C	---	---	0.78	---	---	---	---
TC #1	600S162-43(50)	16.80	2.01C	---	---	0.50	---	---	---	---
TC #2	600S162-43(50)	16.80	2.01C	---	---	0.50	---	---	---	---
Web #1 7	600S162-43(50)	0.97	0.87C	0.331	0.60	0.14	#10-16 FPHSD	0.287	0.87C	4
Web #2 6	600S162-43(50)	7.69	1.00C	0.379	0.74	0.50	#10-16 FPHSD	0.287	1.00C	4
Web #3 5	600S162-43(50)	7.93	0.72C	0.270	0.75	0.37	#10-16 FPHSD	0.287	0.72C	4
Web #4	600S162-43(50)	10.76	1.50T	0.568	1.00	0.29	#10-16 FPHSD	0.287	1.50T	6

Notes:
1. For out-of-plane webs: Mec = eccentric moment, Rm = Reduction factor [AISI 240, Section E: TRUSSES].
2. Truss material: ASTM A653
3. Fastener- Simpson ICC-EC Evaluation Report ESR-3006
4. Truss members checked for Lateral-Torsional Buckling per AISI S100-16, Section F2.1
5. Truss members checked for Distortional Buckling per AISI S100-16, Sections F4, F4.1

Project : 5675 Dunnigan Rd
Location : Lockport, NY 14094
Truss Mark : T1

D3
File: T1

TRUSS CONNECTION DESIGN DATA

E = 29,500 ksi
Fy = 50 ksi (weakest design grade, member or plate)
Fu = 65 ksi (connector plate)
T = 0.045 in (connector plate)
FS = 3.00 (Factor of Safety)

#10-16 FPHSD

Maximum Connector Spacing	= 2.201 in	'AISI, Section I1.2
Minimum Connector Spacing	= 0.570 in	'AISI, Section J4.2
Connector Edge Distance	= 0.285 in	'AISI, Section J4.2

Truss End Bearing:

Fp (allowable end bearing)	= 33,000 psi
Fa (actual end bearing)	= 256 psi

BENDING + WEB CRIPPLING (Sec. H3)

Support # 1 Reaction = 2.50, Pwa = 1.24 | Ratio = 1.71 'Not required, truss web acts as stiffener
Support # 2 Reaction = 2.50, Pwa = 1.24 | Ratio = 1.71 'Not required, truss web acts as stiffener

Project : 5675 Dunnigan Rd
Location : Lockport, NY 14094
Truss Mark : T1

D4
File: T1

LATERAL BRACING DESIGN [LGSEA Tech Note 551e]
Maximum Diagonal Spacing = 20 ft (4 bays)

Bottom Chord Bracing:

Lateral Brace Member	= 150F125-30(33)
Diagonal Brace Member	= 150F125-30(33)
Lateral Brace Spacing	= 8.00 ft
Lateral Wind Force	= 0.220 kip
Lateral Seismic Force	= 0.000 kip
Lateral Chord Force	= 2% * 0.419 * 4 Bays = 0.034 kip
Maximum Chord Force	= 0.220 kip
Diagonal Brace Force	= Maximum Chord Force / Cos(63.43) = 0.492 kip

Lateral Force (Allowable)	= 1.300 kip (Height = 4.000 ft)
Diagonal Force (Allowable)	= 0.667 kip (Height = 8.944 ft)

Connection:
Fastener - #10-16 FPHSD (Va = 0 kip)

Lateral Brace to Bottom Chord	= 0.220 / 0.1603	= 2 each
Diagonal Brace to Bottom Chord	= 0.492 / 0.1603	= 4 each

Top Chord Bracing: not required [flanges sheathed]

Project : 5675 Dunnigan Rd
Location : Lockport, NY 14094
Truss Mark : T1

D5
File: T1

TRUSS SUPPORT DESIGN
** Uplift Load Combination (Truss to Support Connection Design): 0.6Dead + 0.6Wind

Support (Left): Width = 3.5 in
Support Connector = 1 each
Shear (V) = 0.717 kip (Load per connector)
Tension (T) = 0.010 kip (Load per connector)
Compression (C) = 2.495 kip (Load per connector)

Connector - Simpson S/H1
Allowable Load =0.305 kip

Fasteners (Connection to Truss Chord) - #10-16 FPHSD
Number Required = Connector Load / Allowable Fastener Load
Number Required = 0.010/ 0.287 = 3 each

Fasteners (Connection to Support) - #10-16 FPHSD
Number Required = Connector Load / Allowable Fastener Load
Number Required = 0.010/ 0.287 = 3 each

Support (Right): Width = 3.5 in
Support Connector = 1 each
Shear (V) = 0.000 kip (Load per connector)
Tension (T) = 0.010 kip (Load per connector)
Compression (C) = 2.495 kip (Load per connector)

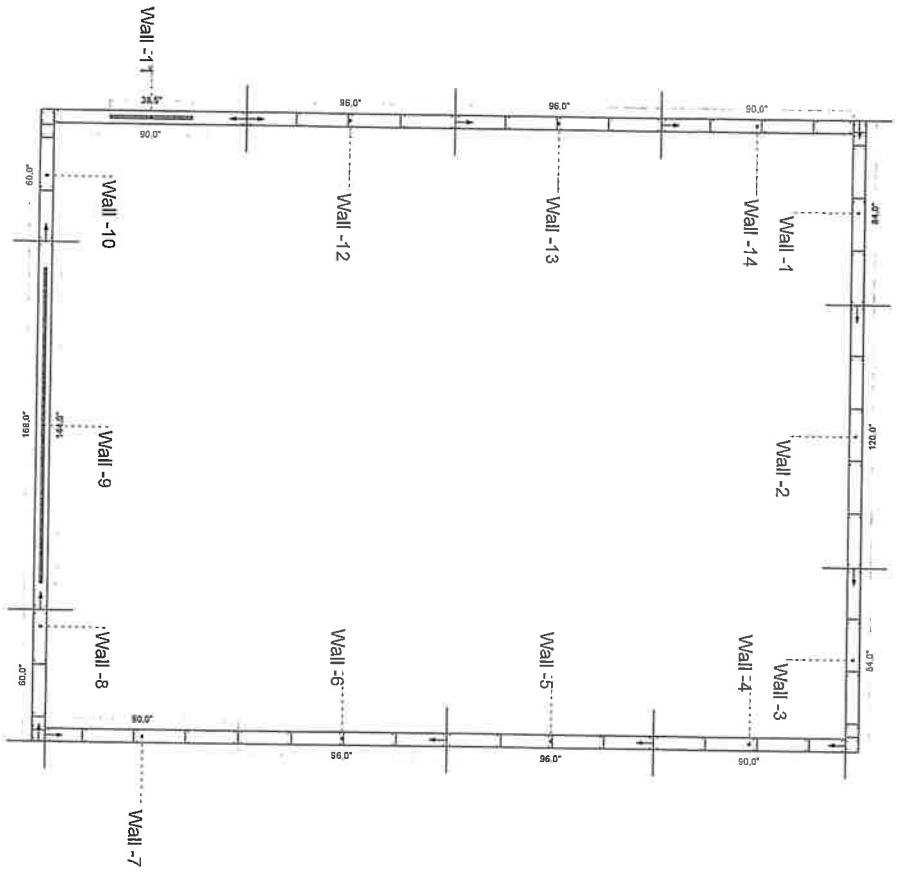
Connector - Simpson S/H1
Allowable Load =0.305 kip

Fasteners (Connection to Truss Chord) - #10-16 FPHSD
Number Required = Connector Load / Allowable Fastener Load
Number Required = 0.010/ 0.287 = 3 each

Fasteners (Connection to Support) - #10-16 FPHSD
Number Required = Connector Load / Allowable Fastener Load
Number Required = 0.010/ 0.287 = 3 each

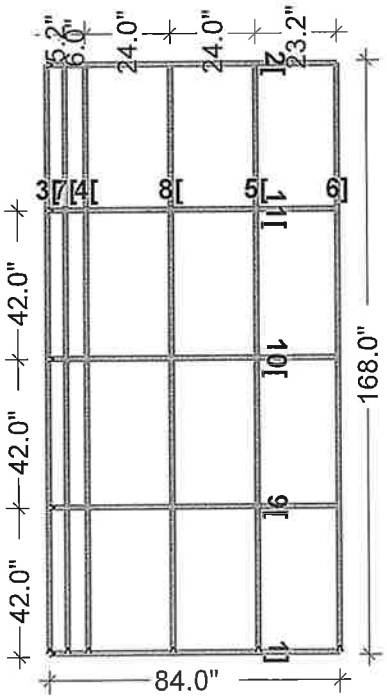
** Note - Number of fasteners may be greater than calculation due to minimum connector requirements

Wall Layout



Drawing	Wall -1		0.0"	Height	168.0"	Length	84.0"	Screws	60

Item ID	QTY	Profile	Desc	Length
1	1	600S162-43	bottom_plat	84.0"
2	1	600S162-43	top_plate	84.0"
3,4,5,6,7,	6	600S162-43	stud	167.8"
9,10,11	3	600S162-43	nog	83.8"
Length(ft)				118.856



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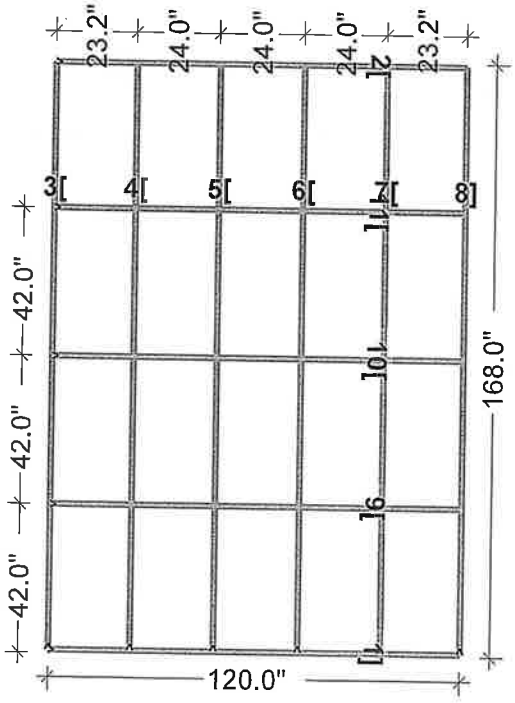
Drawing	Wall -2		0.0"	Height	168.0"	Length	120.0"	Screws	60

Item ID	QTY	Profile	Desc	Length
1	1	600S162-43	bottom_plat	120.0"
2	1	600S162-43	top_plate	120.0"
3,4,5,6,7,	6	600S162-43	stud	167.8"
9,10,11	3	600S162-43	nog	119.8"
Length(ft)				133.857



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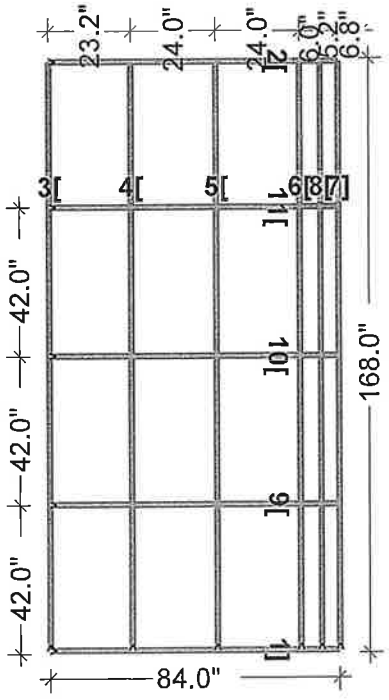
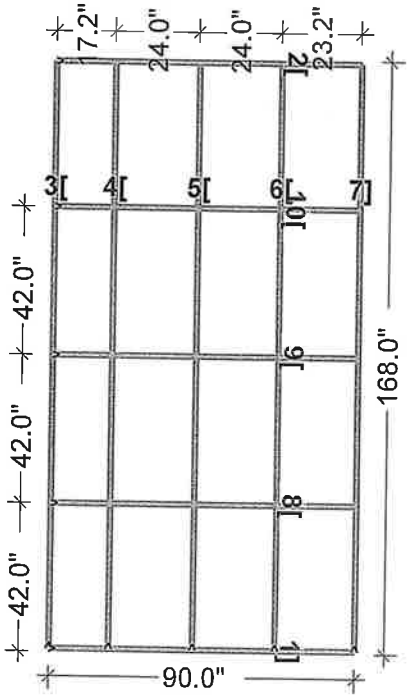
Drawing	Wall -3		0.0"	Height	168.0"	Length	84.0"	Screws	60

Item ID	QTY	Profile	Desc	Length
1	1	600S162-43	bottom_plat	84.0"
2	1	600S162-43	top_plate	84.0"
3,4,5,6,7,	6	600S162-43	stud	167.8"
9,10,11	3	600S162-43	nog	83.8"
Length(ft)				118.856



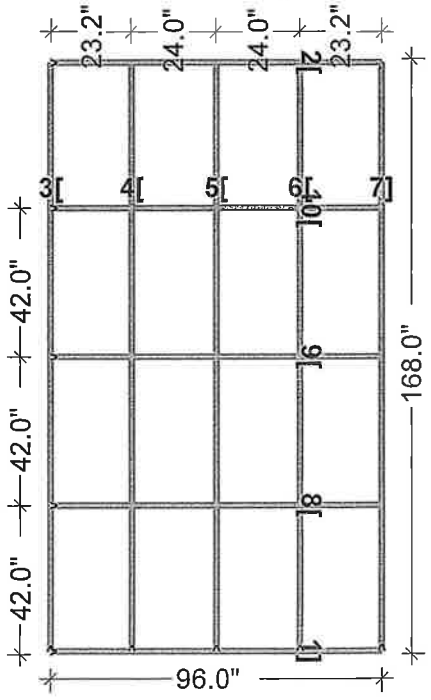
Drawing	Wall -4		0.0"	Height	168.0"	Length	90.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	90.0"
2	1	600S162-54	top_plate	90.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	89.8"
Length(ft)				107.372



Drawing	Wall -5		0.0"	Height	168.0"	Length	96.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	96.0"
2	1	600S162-54	top_plate	96.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	95.8"
Length(ft)				109.872



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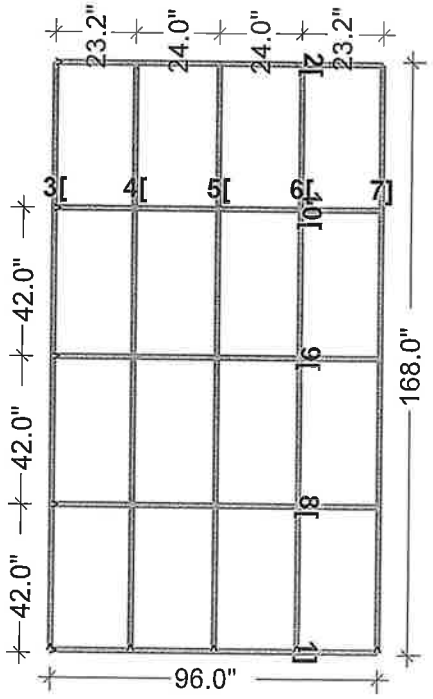
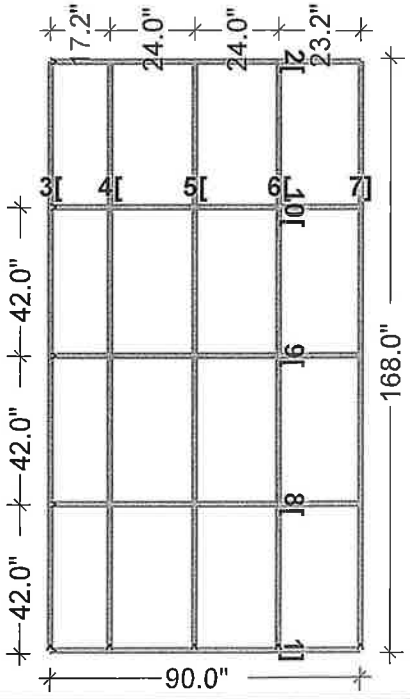
Drawing	Wall -6		0.0"	Height	168.0"	Length	96.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	96.0"
2	1	600S162-54	top_plate	96.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	95.8"
Length(ft)				109.872



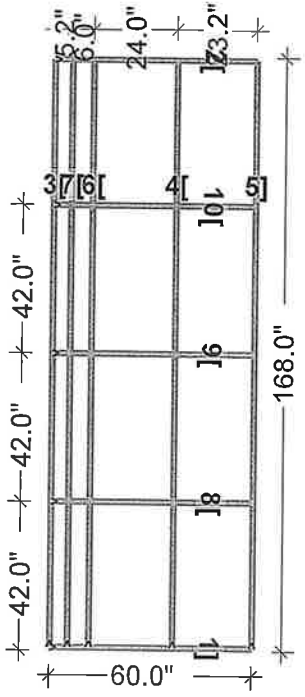
Drawing	Wall -7		0.0"	Height	168.0"	Length	90.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	90.0"
2	1	600S162-54	top_plate	90.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	89.8"
Length(ft)				107.372



Drawing	Wall -8		0.0"	Height	168.0"	Length	60.0"	Screws	50

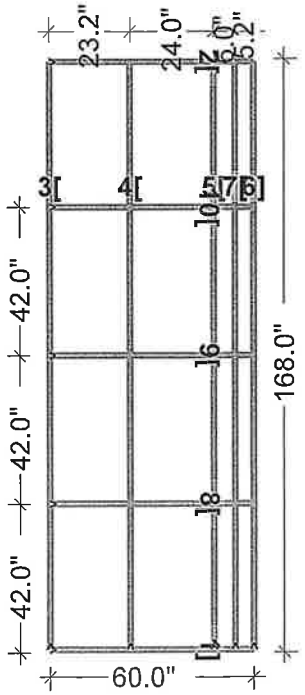
Item ID	QTY	Profile	Desc	Length
1	1	600S162-43	bottom_plat	60.0"
2	1	600S162-43	top_plate	60.0"
3,4,5,6,7	5	600S162-43	stud	167.8"
8,9,10	3	600S162-43	nog	59.8"
Length(ft)				94.871



Generated with FrameBuilder-MRD Light Steel Designer. V5.3.2

Drawing	Wall -10		0.0"	Height	168.0"	Length	60.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-43	bottom_plat	60.0"
2	1	600S162-43	top_plate	60.0"
3,4,5,6,7	5	600S162-43	stud	167.8"
8,9,10	3	600S162-43	nog	59.8"
Length(ft)				94.871



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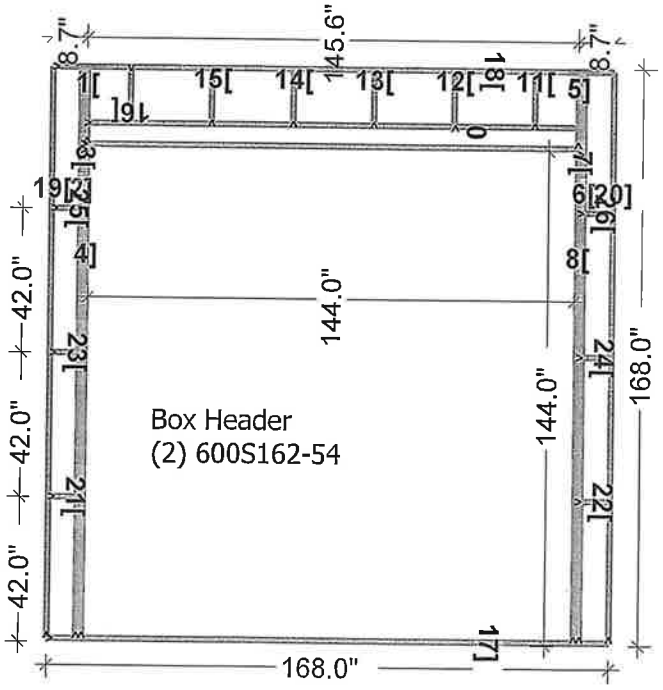
Drawing	Wall -9		0.0"	Height	168.0"	Length	168.0"	Screws	104

Item ID	QTY	Profile	Desc	Length
1,5	2	600S162-54	jack_stud	23.8"
2,6	2	600S162-54	king_stud	167.8"
3,7	2	600S162-54	jamb_cap	~ 3.1"
4,8	2	600S162-54	jack_stud_b	~ 143.7"
9	1	600S162-54	header	~ 150.5"
10	1	600S162-54	added_nog	~ 147.1"
11,12,13,	5	600S162-54	added_stud	~ 17.7"
16	1	600S162-54	added_profi	~ 17.7"
17	1	600S162-54	bottom_plat	168.0"
18	1	600S162-54	top_plate	168.0"
19,20	2	600S162-54	stud	167.8"
21,22,23,	6	600S162-54	nog	11.8"
Length(ft)				151.938



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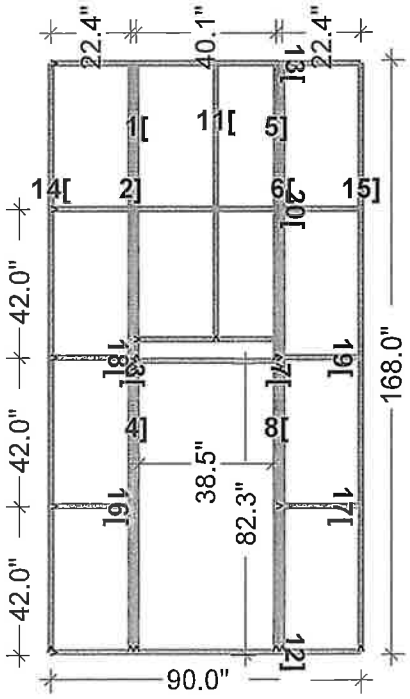
Drawing	Wall -11		0.0"	Height	168.0"	Length	90.0"	Screws	86

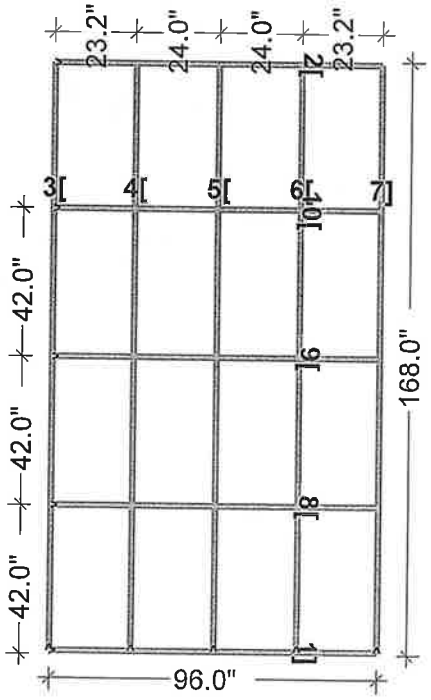
Item ID	QTY	Profile	Desc	Length
1,5	2	600S162-54	jack_stud	~ 85.5"
2,6	2	600S162-54	king_stud	167.8"
3,7	2	600S162-54	jamb_cap	~ 3.1"
4,8	2	600S162-54	jack_stud_b	~ 82.0"
9	1	600S162-54	header	~ 45.0"
10	1	600S162-54	added_nog	~ 41.6"
11	1	600S162-54	added_stud	~ 79.5"
12	1	600S162-54	bottom_plat	90.0"
13	1	600S162-54	top_plate	90.0"
14,15	2	600S162-54	stud	167.8"
16,17,18,	4	600S162-54	nog	~ 25.6"
20	1	600S162-54	nog	89.8"
Length(ft)				129.216



Box Header
(2)600S162-54

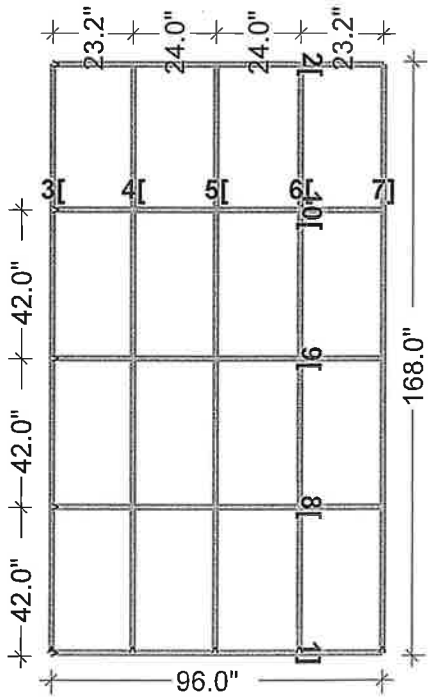
Drawing	Wall -12		0.0"	Height	168.0"	Length	96.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	96.0"
2	1	600S162-54	top_plate	96.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	95.8"
Length(ft)				109.872



Drawing	Wall -13		0.0"	Height	168.0"	Length	96.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	96.0"
2	1	600S162-54	top_plate	96.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	95.8"
Length(ft)				109.872



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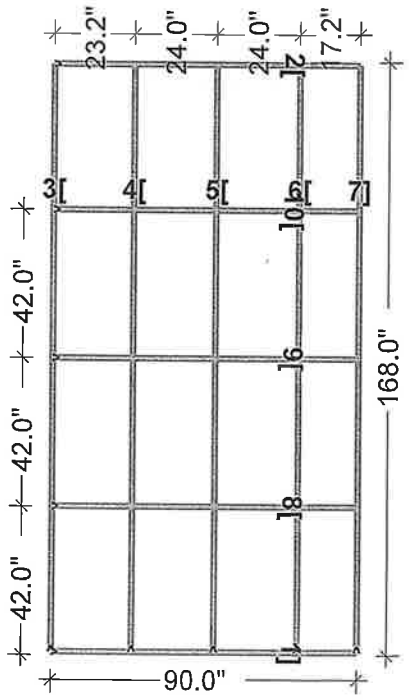
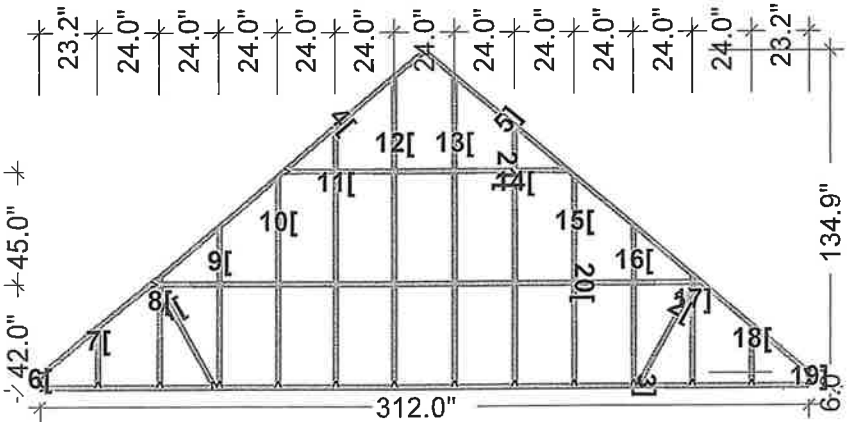
Drawing	Wall -14		0.0"	Height	168.0"	Length	90.0"	Screws	50

Item ID	QTY	Profile	Desc	Length
1	1	600S162-54	bottom_plat	90.0"
2	1	600S162-54	top_plate	90.0"
3,4,5,6,7	5	600S162-54	stud	167.8"
8,9,10	3	600S162-54	nog	89.8"
Length(ft)				107.372



Drawing	Gable Truss -1		168.0"	Height	6.0"	Length	312.0"	Screws	102

Item ID	QTY	Profile	Desc	Length
1,2	2	600S162-43	added_profi	46.4"
3	1	600S162-43	bottom_plat	312.0"
4	1	600S162-43	top_plate_1	~ 203.2"
5	1	600S162-43	top_plate_2	~ 203.2"
6,19	2	600S162-43	stud	~ 6.2"
7,18	2	600S162-43	stud	~ 25.5"
8,17	2	600S162-43	stud	~ 45.5"
9,16	2	600S162-43	stud	~ 65.5"
10,15	2	600S162-43	stud	~ 85.5"
11,14	2	600S162-43	stud	~ 105.5"
12,13	2	600S162-43	stud	~ 125.5"
20	1	600S162-43	nog	~ 224.3"
21	1	600S162-43	nog	~ 116.3"
Length(ft)				172.51



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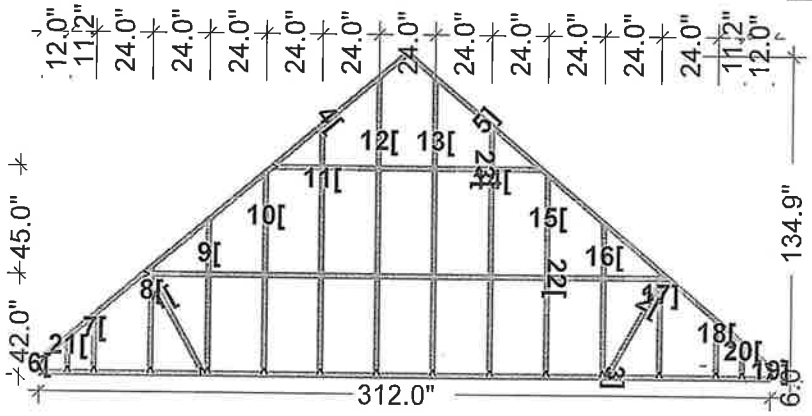
Drawing	Gable Truss -2		168.0"	Height	6.0"	Length	312.0"	Screws	110

Item ID	QTY	Profile	Desc	Length
1,2	2	600S162-43	added_profi	46.4"
3	1	600S162-43	bottom_plat	312.0"
4	1	600S162-43	top_plate_1	~ 203.2"
5	1	600S162-43	top_plate_2	~ 203.2"
6,19	2	600S162-43	stud	~ 6.2"
7,18	2	600S162-43	stud	~ 25.5"
8,17	2	600S162-43	stud	~ 45.5"
9,16	2	600S162-43	stud	~ 65.5"
10,15	2	600S162-43	stud	~ 85.5"
11,14	2	600S162-43	stud	~ 105.5"
12,13	2	600S162-43	stud	~ 125.5"
20,21	2	600S162-43	stud	~ 16.2"
22	1	600S162-43	nog	~ 224.3"
23	1	600S162-43	nog	~ 116.3"
Length(ft)				175.205



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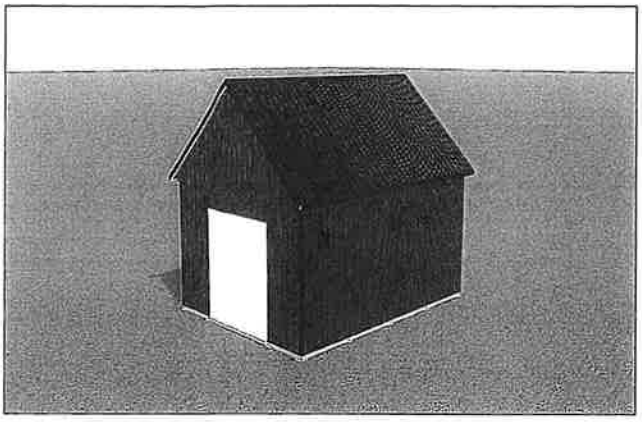
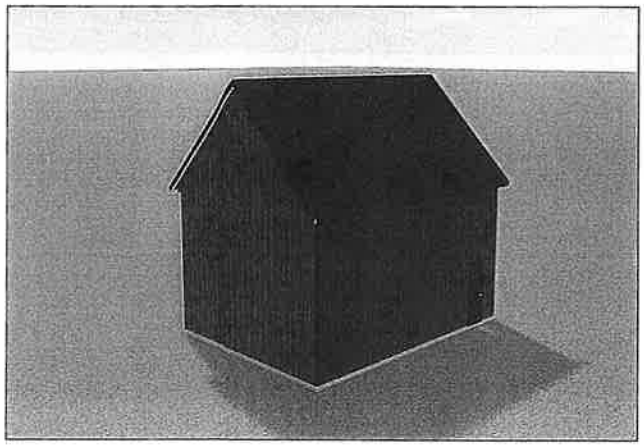
SUMMARY

Width	24'
Length	32'
Ceiling Height	14'
Overhangs	1'
Roof Pitch	10 / 12

JOB INFORMATION

Project Name	Harman 24x32-14

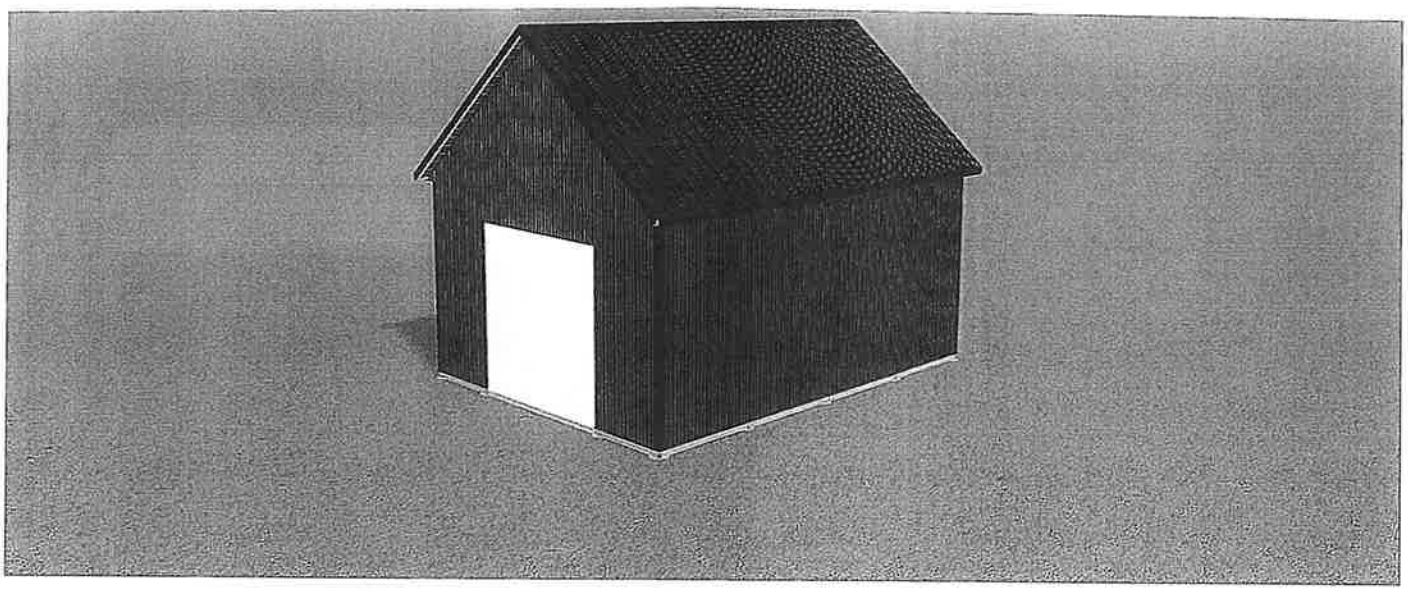




ALL METAL WORKS

Job: Harman 24x32-14

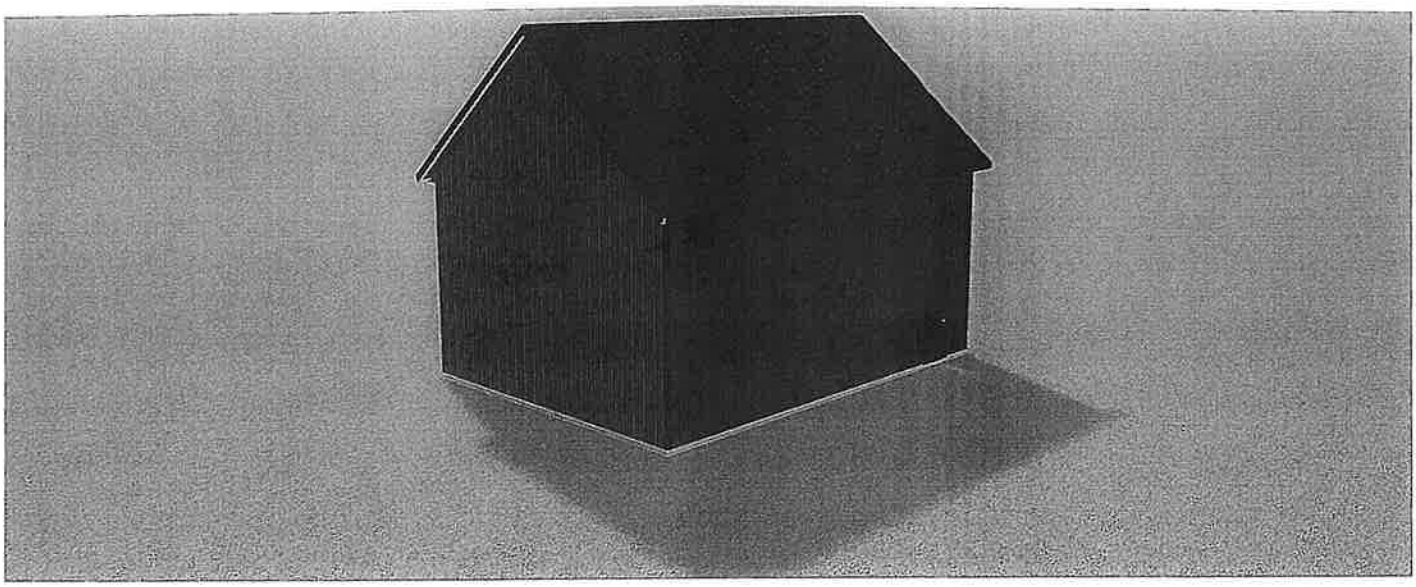
Iso 1



ALL METAL WORKS

Job: Harman 24x32-14

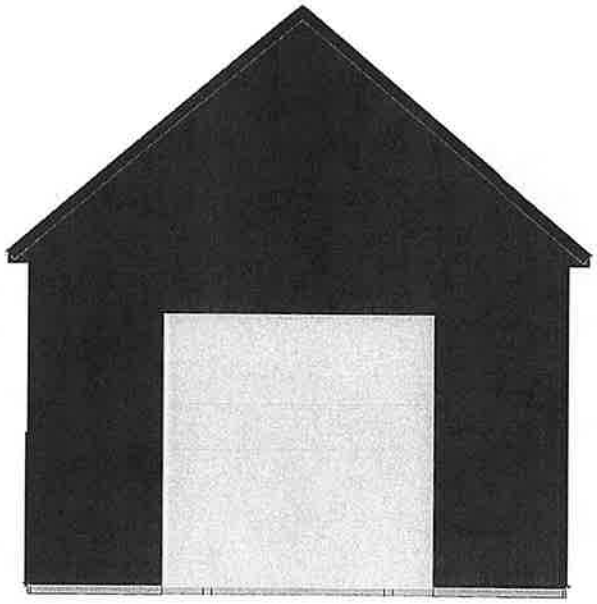
Iso 2



ALL METAL WORKS

Job: Harman 24x32-14

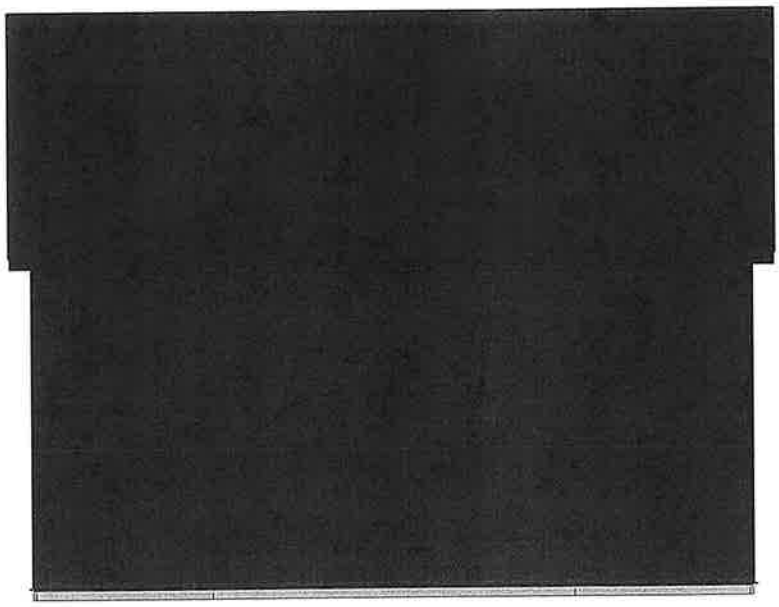
Front Elevation



ALL METAL WORKS

Job: Harman 24x32-14

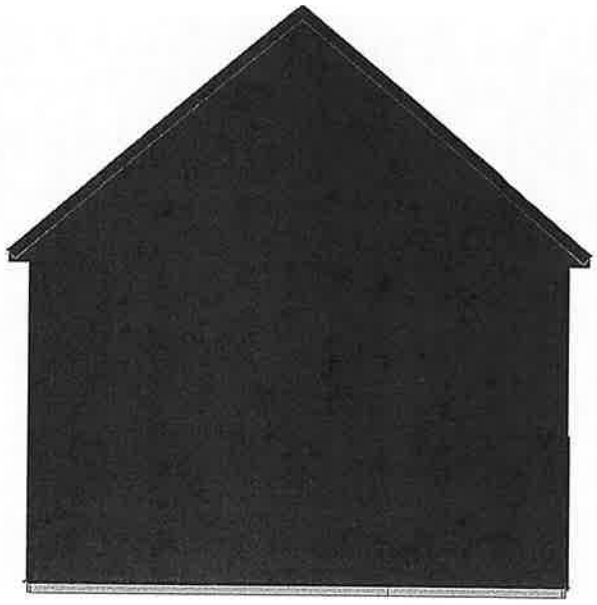
Left Elevation



ALL METAL WORKS

Back Elevation

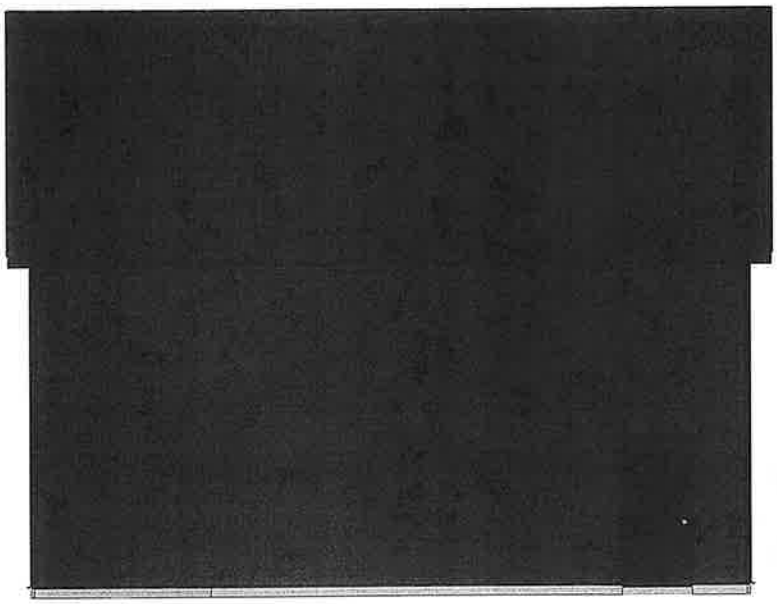
Job: Harman 24x32-14



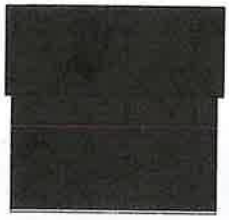
ALL METAL WORKS

Job: Harman 24x32-14

Right Elevation

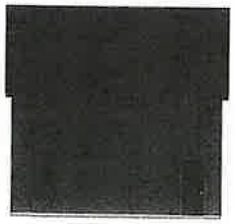


FRONT



LEFT

BACK



RIGHT

ALL METAL WORKS

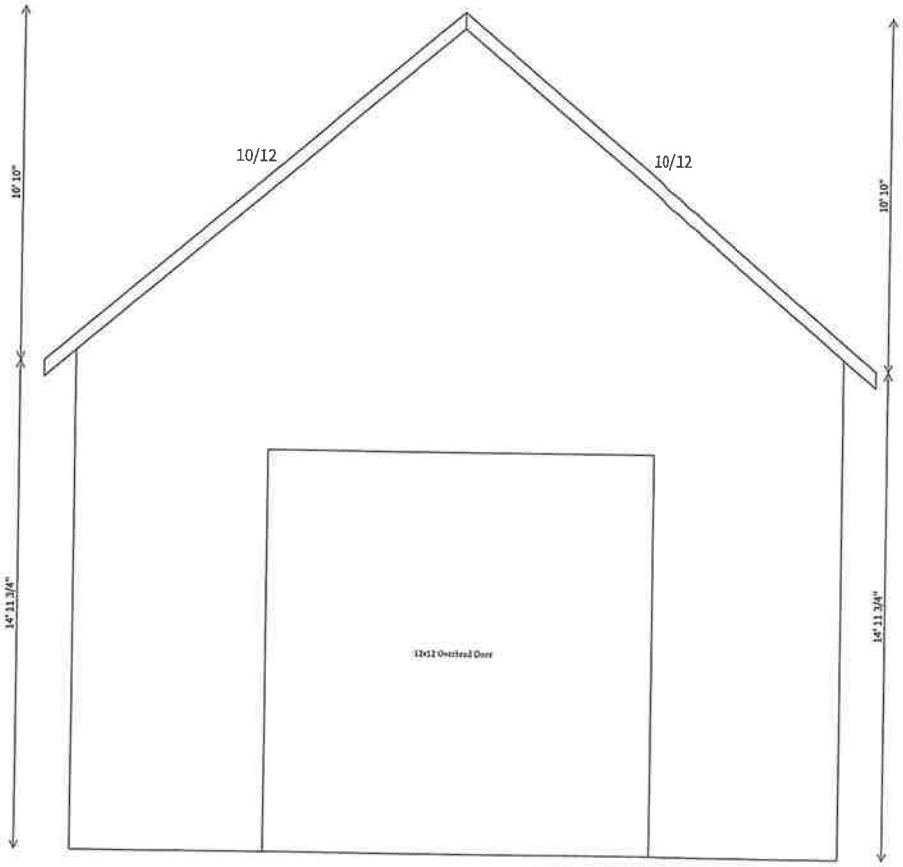
Job: Harman 24x32-14

Left Elevation

ALL METAL WORKS

Front Elevation

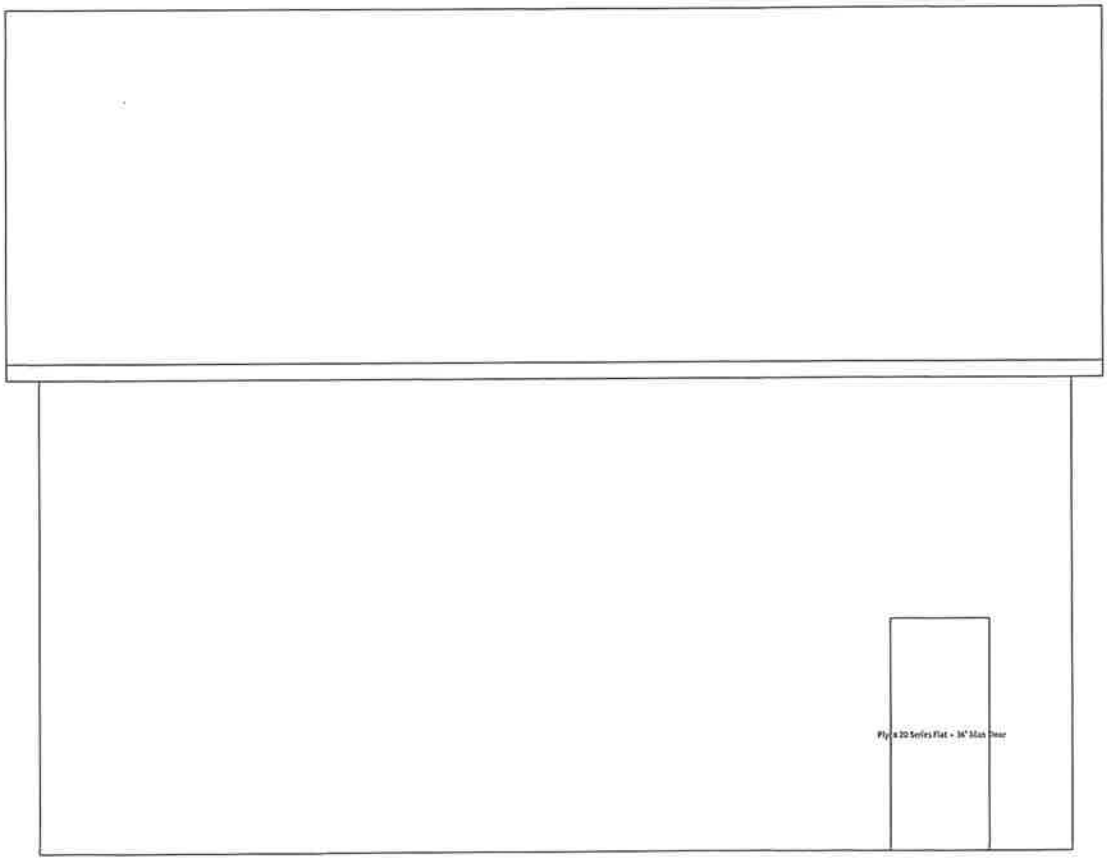
Job: Harman 24x32-14



ALL METAL WORKS

Job: Harman 24x32-14

Right Elevation



ALL METAL WORKS

Job: Harman 24x32-14

Back Elevation